



GEOSCIENCE TECHNOLOGIES, INC.

GEOTECHNICAL INVESTIGATION REPORT

PROPOSED RESIDENTIAL COMPLEX PROJECT

Brgy. Coloong #1, Valenzuela City

Submitted to:

**Technical Assistance Movement for People and Environment
(TAMPEI)**

234-A, Tandang Sora Avenue
Quezon City 1116

JUNE 2022

GEOSCIENCE TECHNOLOGIES, INC.

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Brgy. Coloong #1, Valenzuela City

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1.0 INTRODUCTION

This report contains an outline of field and laboratory methods involving both simple and complex techniques that were accomplished for site characterization that are necessary to formulate geotechnical study for the engineering design of the proposed type of project.

This includes evaluation of the site for idealized condition and its implications with physical and engineering realities that are influential in proper selection and construction of any foundations / substructures on the area.

2.0 FIELD TEST

2.1 Drilling in Soils

The boring was accomplished using a rotary-drilling rig. In between sampling sections, the hole was advanced using the rotary wash method. Standard Penetration Test (SPT) is carried out by using a standard split-spoon sampler, mounted on a drive rod of sufficient strength to prevent whipping from blows delivered by 140 pound (63.5 kg) hammer free-falling from a height of 30 in. (76 cm). The value of N is reported as the resistance to penetration. It is the number of blows required to drive the tube to the last 300 mm (12 in) of penetration distance.

After the sample and tube are brought to the surface and separated, the sample is removed from the tube and properly preserved and sealed using a moisture tight plastic bag for further testing in the laboratory.

Correlation of SPT data with other soil parameters have been developed for estimates of stiffness of a soil and is a very useful supplementary classification as shown in tables below:

CONSISTENCY CLASSIFICATION FOR FINE-GRAINED SOILS

(Terzaghi & Peck, 1969)

Classification	SPT, N	Undrained Shear Strength, s_u (kPa)
Very soft	<2	<12
Soft	2 - 4	12 - 25
Medium stiff	4 - 8	25 - 50
Stiff	8 - 15	50 - 100
Very Stiff	15-30	100 - 200
Hard	>30	>200

RELATIVE DENSITY CLASSIFICATION FOR COARSE-GRAINED SOILS

(Terzaghi & Peck, 1969)

Classification	SPT, N	Relative Density, D_r (%)
Very loose	<4	0 – 15
Loose	4 – 10	15 – 35
Medium dense	10 – 30	36 – 65
Dense	30 – 50	65 – 85
Very dense	>50	85 – 100

3.0 LABORATORY TEST

Selected soil samples were subjected to the following specific tests.

3.1 Soil Particle Size Analysis (ASTM D422, ASTM D1140)

The size and quantity of individual particles found in particular soil is indicative of the performance characteristics of the soil. The percentage by weight of the material passing through each succession sieve is recorded.

3.2 The Atterberg Limits (ASTM D4318)

The liquid limit and the plastic limit tests define the upper and lower moisture content points at which a particular soil ceases to perform as a plastic. The use of this test is restricted to cohesive soils.

3.3 Moisture Content of Soils (ASTM D2216, ASTM D4959)

It is based on the weight of the water in the soil. This indicates imperative behavior of different soil types at various levels of moisture.

3.4 Standard Classification of Soils for Engineering Purposes (ASTM- D2487, ASTM D3282)

Based on the results of visual observations and prescribed laboratory tests, a soil is catalogued according to the basic soil groups, assigned a group symbol(s) and name and thereby classified. This standard classifies soils from any geographic location into categories representing the results of prescribed laboratory tests to determine the particle-size characteristics, the liquid limit, and the plasticity index.

The various groupings of the classification system have been devised to correlate in a general way with the engineering behavior of soils.

4.0 GENERAL GEOLOGY & GEOMORPHOLOGY



¹ Valenzuela is located at about 14 km (8.7 mi) north of country's capital, Manila. Valenzuela is bordered in the north by the town of Obando and the city of Meycauayan in Bulacan, the city of Navotas in the west, Malabon in the south and Quezon City and northern portion of Caloocan in the east.

¹ Wikipedia

The highest elevation point is 38 meters (125 ft) above sea level. Having a surface gradient of 0.55% and a gentle slope, hilly landscape is located in the industrial section of the city in Canumay. The average elevation point is 2 meters (6.6 ft) above sea level.

The Luzon Central plain is composed of middle tertiary sediments and Quaternary pyroclastics and lava flows, unconformably overlying Cretaceous to Lower Tertiary basement rocks. Post-basement stratigraphy on the east is distinct from that on the west.

In general, recent alluvial deposits probably derived from the Guadalupe Formation cover a considerable portion of Metro Manila, particularly the plain areas. They are composed of silts, sands and unconsolidated very poorly consolidated and unsorted pebbles, cobbles and small boulders of the underlying rocks. Gervacio (1968) disclosed two separate depositional environments for these alluvial strata; the Manila Deltaic Plain and the Marikina Valley Alluvial Plain. The thickness of the alluvial deposit varies accordingly from place to place. In the deltaic areas, the deposits measure over 50 meters thick near the coast and thin-out towards the east. In the Marikina plain, it ranges from 30 meters to as much as 130 meters.

The Pleistocene Guadalupe Formation consisting of massive and thick sequence of volcanic and sedimentary rock constitutes a vast portion of Metro Manila, extending from Quezon City and Novaliches in the north to as far as Cavite in the south. Tectonic development and vulcanism accompanied by wide fluctuations of sea level during the Late Tertiary to Quaternary periods greatly influences not merely the geology and geomorphic aspects but also the structural elements. Teves and Gonzales (1950) adopt the name Guadalupe Formation with two members: a lower Alat Conglomerate and an Upper Diliman Tuff member. The formation unconformably overlies Miocene rocks.

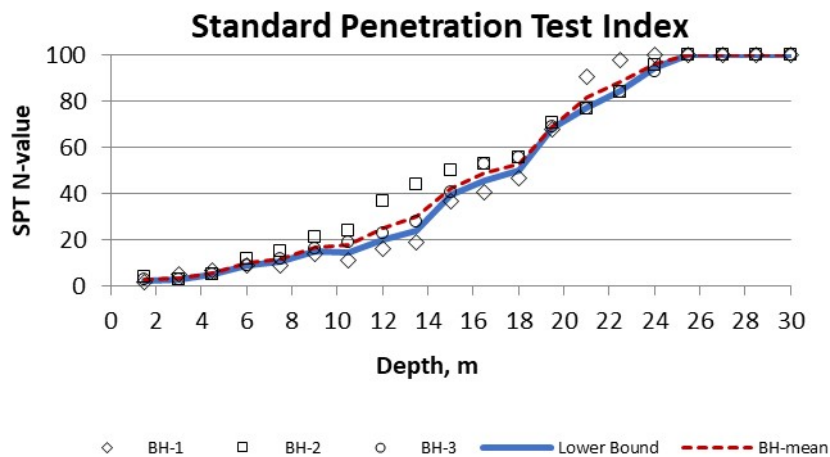
Overlying the Diliman Tuff is a sequence of unconsolidated fluvial, deltaic and marine deposits to which Purser and Diomampo (1995) proposed the name Manila Formation. The sequence is believed to have been laid down during Holocene time. The unconsolidated deposits consist of clay, silt, gravelly sand and tuffaceous silt.

The most significant structures which have important bearing to the Metro Manila are the existence of two parallel northeast trending Valley Fault Systems. These

two faults enclosed the Marikina Valley and the adjacent municipalities of Rodriguez (Montalban), San Mateo, Antipolo and the eastern sections of Metro Manila.

5.0 SUBSOIL CONDITION

Based on the three soil borings conducted at the site, fine-grained soils of silt and clay (MH, CL, CH) are occupying the upper zone, at varying thickness of at least 6m depth along the influence of BH-1 and at least 19.5m depth along locations of BH-2 and BH-3. The soils are varying from medium to high plasticity conditions. Sandy soils (SM, SP, SW-SM) are located subsequently through 30m extent depth of exploration. The site area is low-lying with seawater/groundwater level varying from +3.71m to +3.94m depth with respect to boring locations at the time of observation.



Standard penetration test is indicating very soft to medium stiff silt and clay (MH, CL) occupying the upper 4.5m depth with SPT N-values varying from 2 to 7. Medium stiff to stiff silt and clay including loose sand are extending subsequently to 7.5m depth as implied by SPT N-values increasing with depth from 5 to 15.

Very stiff to hard clay (CL, CH) and medium dense sand (SW-SM, SP) are extending from 7.5m to 13.5m depth with SPT N-values further increasing with depth from 14 to 44. Dense sand and hard clay were observed between 13.5m to 19.5m depth with SPT N-values increasing with depth from 37 to 71.

Consistently very dense sandy soil is supporting the area beyond 19.5m depth as reflected by SPT N-values ≥ 77 with profound refusals (≥ 100) through deeper levels.

6.0 DISCUSSIONS AND RECOMMENDATIONS

6.1 Structural Foundation

Considering the site location for the intended residential complex, this will require foundation through the use of embankments including the use of piles for appropriate load transfer. Embankment shall anticipate time-dependent consolidation of soils under load application noting the influencing generally soft to stiff fine-grained soils occupying the upper 8m depth.

The use of pile (either driven or drilled shaft including micropile) shall be extended ≥ 18 m depth along locations for fixity.

6.2 Ultimate Resistance of the Underlying Soil Deposit for Offshore Structure

For the offshore location, frictional and end bearing resistance of the underlying soil deposits are given in ultimate values.

Ref: BH-1

Influence Depth	Ultimate Skin Resistance, kPa	Ultimate Uplift Skin Resistance, kPa	Tip Resistance, kPa
Upper 3m	-	-	
3m to 6m	6	6	40 kPa (≥ 3 m)
6m to 9m	14	14	150 kPa (≥ 6 m)
9m to 12m	22	15	830 kPa (≥ 9 m)
12m to 15m	29	22	1140 kPa (≥ 12 m)
15m to 18m	37	32	3600 kPa (≥ 15 m)
18m to 21m	44	43	4300 kPa (≥ 18 m)
21m to 24m	63	52	4300 kPa (≥ 21 m)
24m to 27m	87	62	
>27m	100	72	

Ref: BH-2&3

Influence Depth	Ultimate Skin Resistance, kPa	Ultimate Uplift Skin Resistance, kPa	Tip Resistance, kPa
Upper 3m	-	-	
3m to 6m	6	6	40 kPa (≥3m)
6m to 9m	13	13	150 kPa (≥6m)
9m to 12m	21	21	190 kPa (≥9m)
12m to 15m	28	28	230 kPa (≥12m)
15m to 18m	35	35	270 kPa (≥15m)
18m to 21m	44	44	320 kPa (≥18m)
21m to 24m	63	52	4300 kPa (≥21m)
24m to 27m	87	62	
>27m	100	72	

6.3 Elastic Parameters

For the supporting soil on the area, the elastic parameters are as follows,

Soil Depth	Young's Modulus (Es), MPa	Poisson's ratio, ν
Upper 5m	8 000	0.5
5m to 8m	15 000	
> 8m	50 000	

6.4 Pile Capacity

Results of effective stress method of analysis using the combined friction and end bearing resistance of the soil are given in safe values for bored pile/ drilled shaft sections including smaller dimensions:

Reference: BH-1

PILE SIZE	Safe Pile Axial Capacity in kN [Factor of Safety =3.0]					
	≥ 15M	≥ 18M	≥ 21M	≥ 24M	≥ 27M	≥ 30M
0.20m dia.	270	390	520	640	760	880
0.30m dia.	420	600	820	1010	1180	1360
0.40m dia.	580	840	1150	1390	1620	1860
0.60m dia.	940	1340	1860	2210	2570	2930
0.90m dia.	1570	2210	3090	3620	4160	4700
1.00m dia.	1800	2520	3540	4140	4540	4330
1.20m dia.	2290	3190	4520	5240	5950	6670

Reference: BH-2 & BH-3

PILE SIZE	Safe Pile Axial Capacity in kN [Factor of Safety =3.0]					
	≥ 15M	≥ 18M	≥ 21M	≥ 24M	≥ 27M	≥ 30M
0.20m dia.	150	190	300	410	530	650
0.30m dia.	250	300	480	660	840	1020
0.40m dia.	350	420	680	920	1170	1410
0.60m dia.	590	690	1170	1530	1890	2240
0.90m dia.	1010	1180	2060	2590	3130	3670
1.00m dia.	1260	1350	2400	3000	3590	4190
1.20m dia.	1630	1740	3150	3860	4580	5300

6.5 Seismic Considerations

The area is categorized as under *Seismic Zone 4* with Seismic Source Type A as influenced by Valley Fault System, a group of dextral strike-slip faults², extending from the northern portion of Quezon City to the western side of Laguna de Bay extending south to Tagaytay Ridge in Cavite (Gervacio, 1968). The west fault is capable of producing large-scale earthquakes on its active phases with a magnitude of 7 or higher.

Specific *Soil Profile Type*, S_E , shall be used for foundations on the upper influencing soil materials.

Near-Source Factor values of $N_a=1.0$ and $N_v=1.0$ shall be considered as per NSCP 2015 Tables 208-5 and 208-6.

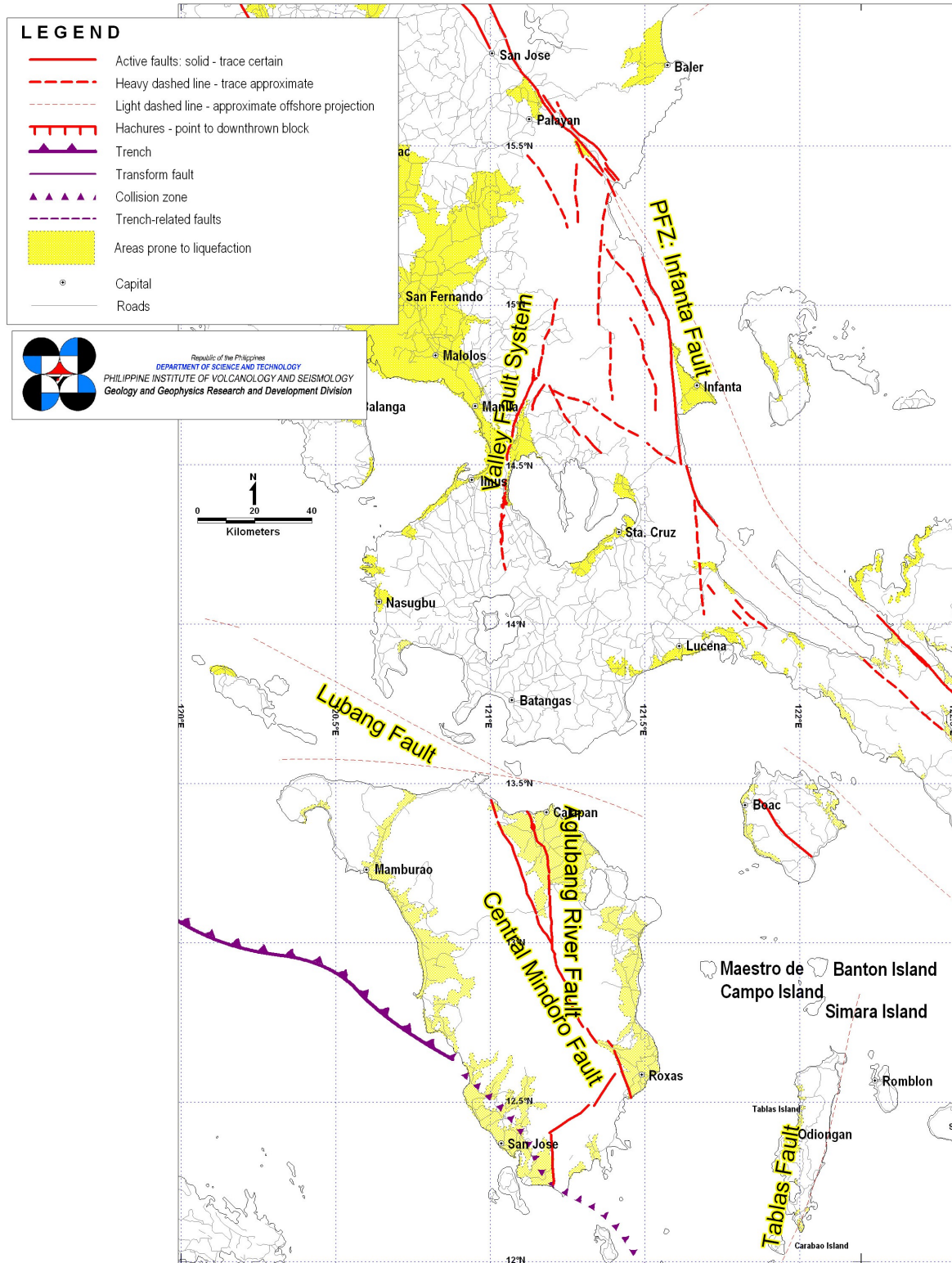
² Rimando & Knuepfer, Neotectonic of Marikina Valley Fault System and Tectonic Framework of Structures in Northern and Central Luzon Philippines.

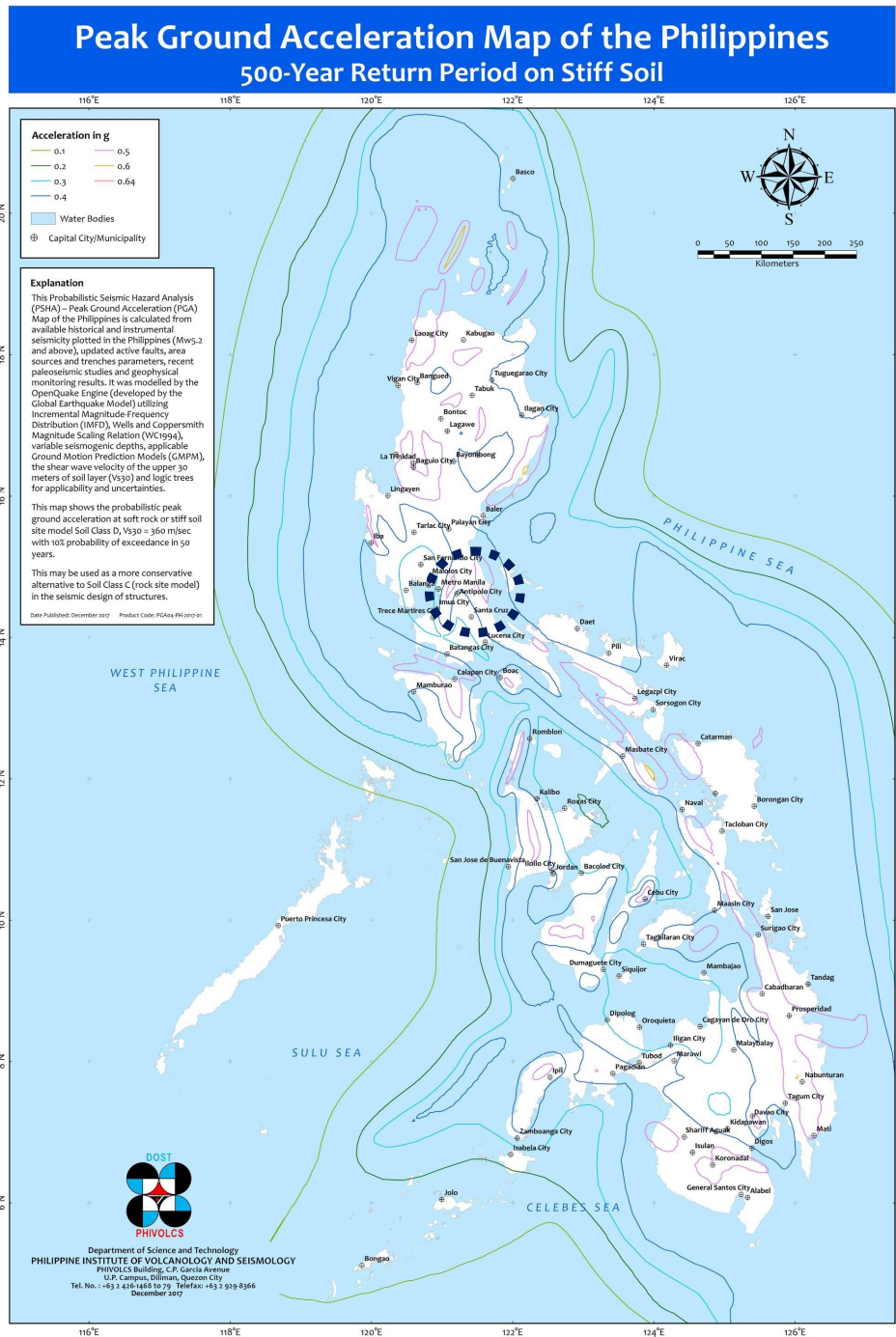
³Seismic Source Relative to Site Location



³ faultfinder.phivolcs.dost.gov.ph

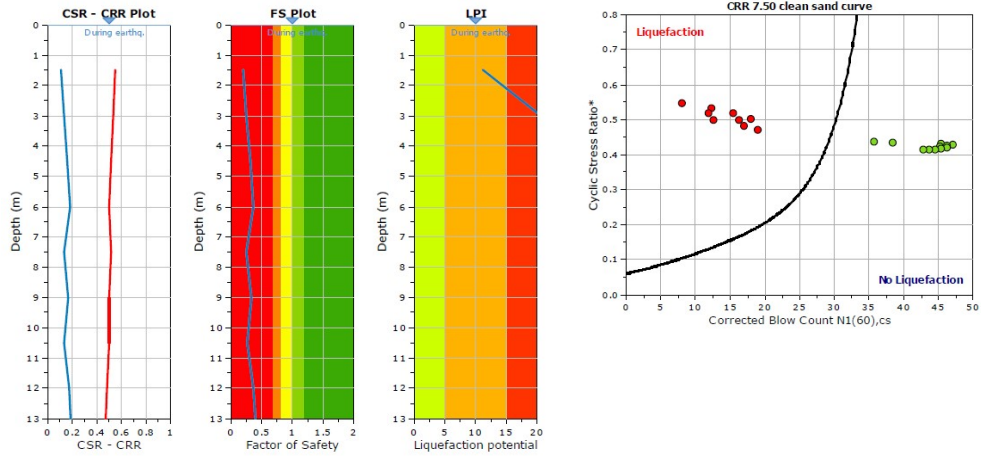
Active Faults and Liquefaction Susceptibility Map of Region IV-A





Following Idriss and Boulanger (2014) and utilizing LiqSVs software for analysis, liquefaction potential is at very high risk for the location. Peak ground acceleration of 0.40g has been used for 500-year return period for Vs30 stiff soil model as per Philippine Earthquake Model (Phivolcs, 2017).

Borehole 1



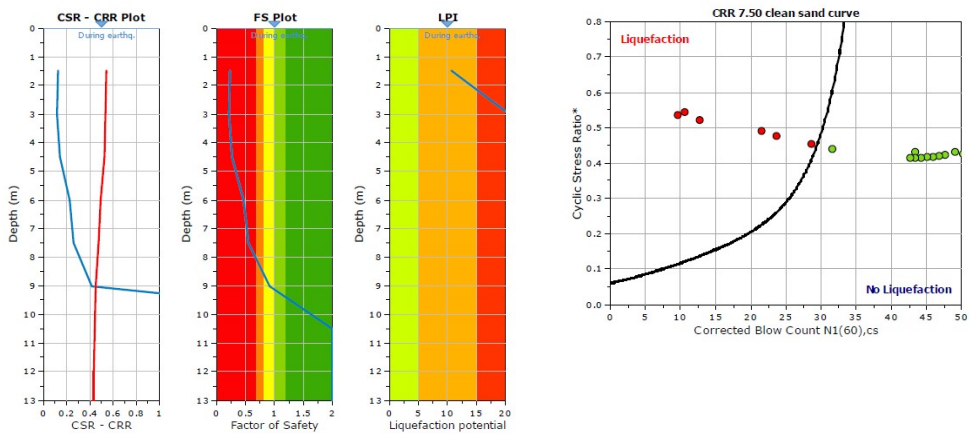
F.S. color scheme

- Red: Almost certain it will liquefy
- Orange: Very likely to liquefy
- Yellow: Liquefaction and no liq. are equally likely
- Green: Unlikely to liquefy
- Dark Green: Almost certain it will not liquefy

LPI color scheme

- Red: Very high risk
- Orange: High risk
- Yellow: Low risk

Borehole 2



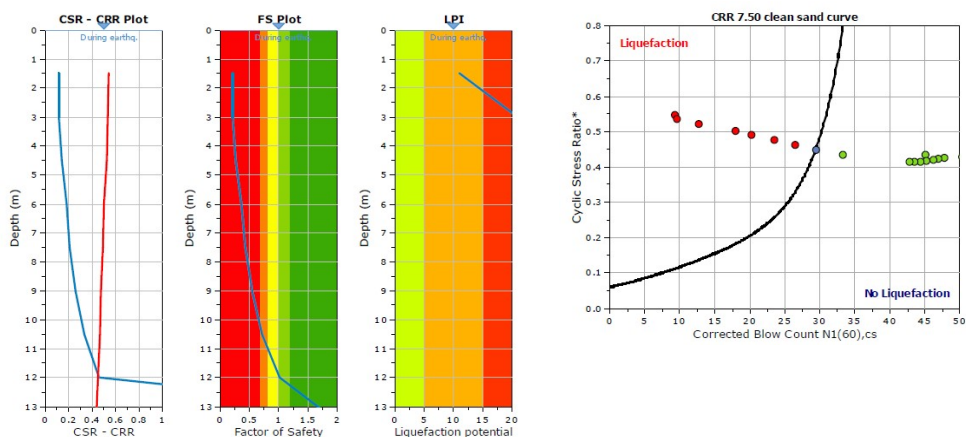
F.S. color scheme

- Red: Almost certain it will liquefy
- Orange: Very likely to liquefy
- Yellow: Liquefaction and no liq. are equally likely
- Green: Unlikely to liquefy
- Dark Green: Almost certain it will not liquefy

LPI color scheme

- Red: Very high risk
- Orange: High risk
- Yellow: Low risk

Borehole 3



F.S. color scheme

- Red: Almost certain it will liquefy
- Orange: Very likely to liquefy
- Yellow: Liquefaction and no liq. are equally likely
- Green: Unlikely to liquefy
- Dark Green: Almost certain it will not liquefy

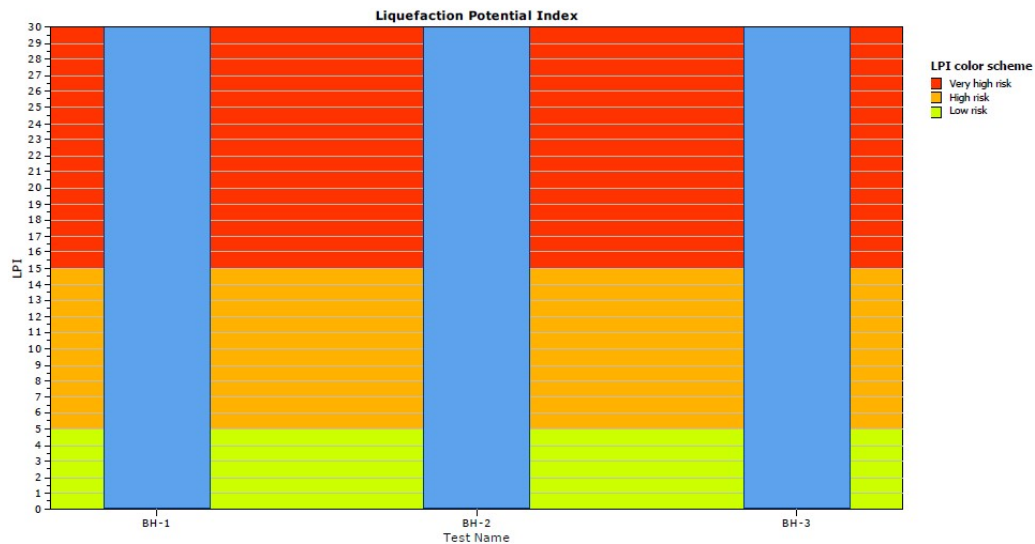
LPI color scheme

- Red: Very high risk
- Orange: High risk
- Yellow: Low risk

Typical Range for liquefaction potential according to Iwazaki

$I_L = 0.00$ - No liquefaction
 I_L between 0.00 and 5 - Liquefaction not probable
 I_L between 5 and 15 - Liquefaction probable
 $I_L > 15$ - Liquefaction certain

Summary calculation report for liquefaction



7.0 LIMITATIONS

Limitations must be acknowledged and the results analyzed in conjunction with the geological information for this specific area of concern. Furthermore, practical observations of the subsoil should be made during construction of every structure on the area.

Prepared by:

Geoscience Technologies, Inc.

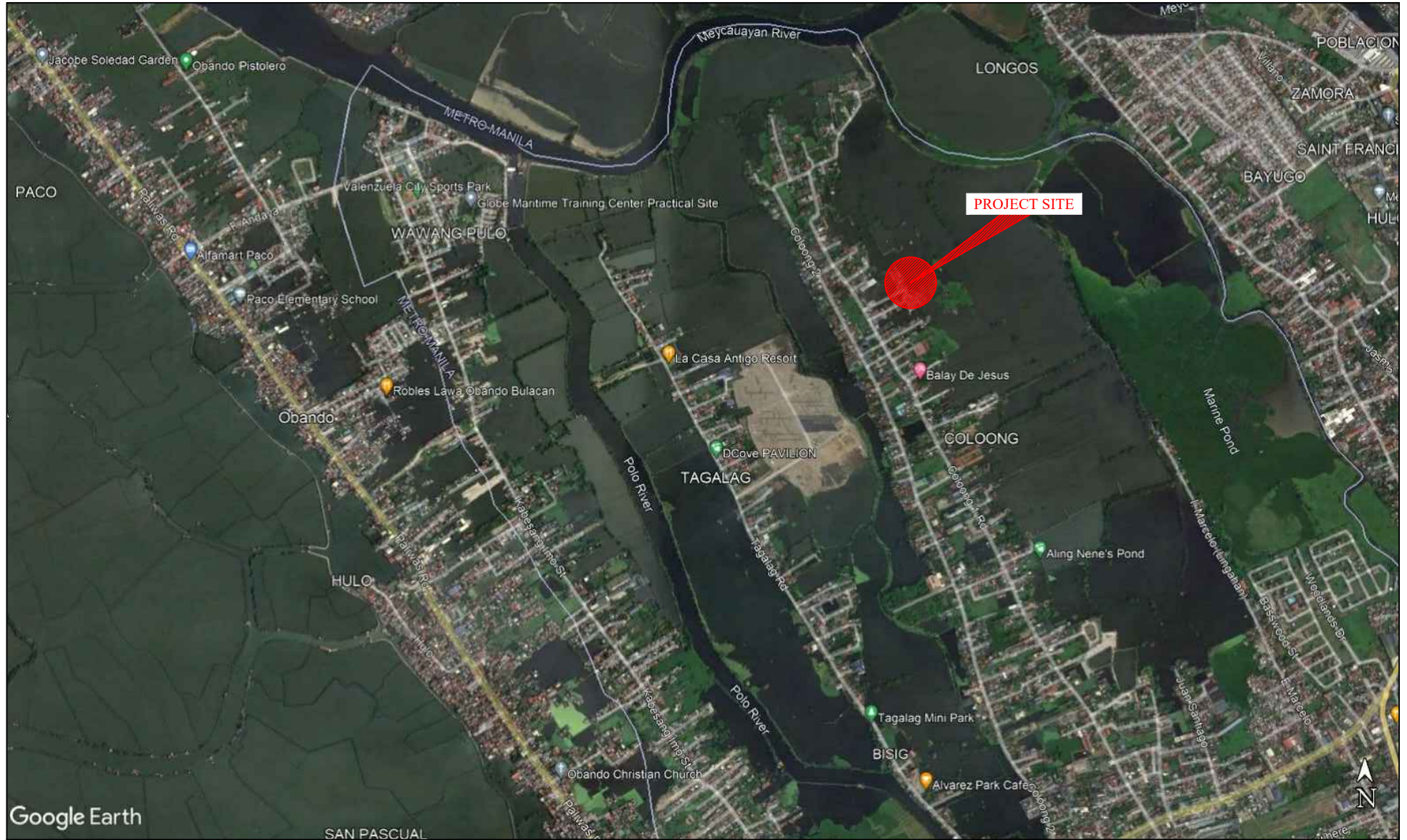
CLIPPER N. BESIN

Civil-Geotechnical Engineer
PRC No. 0078265
PTR No. 29397523 issued on
January 03, 2022 @ San Juan, Batangas

21 June 2022

A P P E N D I C E S

VICINITY MAP



GEOSCIENCE TECHNOLOGIES, INC.
 Rm. 207 Makati Executive Tower II, Cityland Square
 Dela Rosa St., Pio del Pilar, Makati City, Tel. No.: (02) 856-3174

PROJECT NAME AND LOCATION :
PROPOSED RESIDENTIAL COMPLEX PROJECT
 Brgy. Coloong #1, Valenzuela City

SHEET CONTENT :
VICINITY MAP

SHEET NO.
 1 of 1

BOREHOLE LOCATION PLAN



GEOSCIENCE TECHNOLOGIES, INC.
Rm. 207 Makati Executive Tower II, Cityland Square
Dela Rosa St., Pio del Pilar, Makati City, Tel. No.: (02) 856-3174

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Brgy. Coloong #1, Valenzuela City

SHEET CONTENT :
BOREHOLE LOCATION PLAN

SHEET NO.
1 of 1

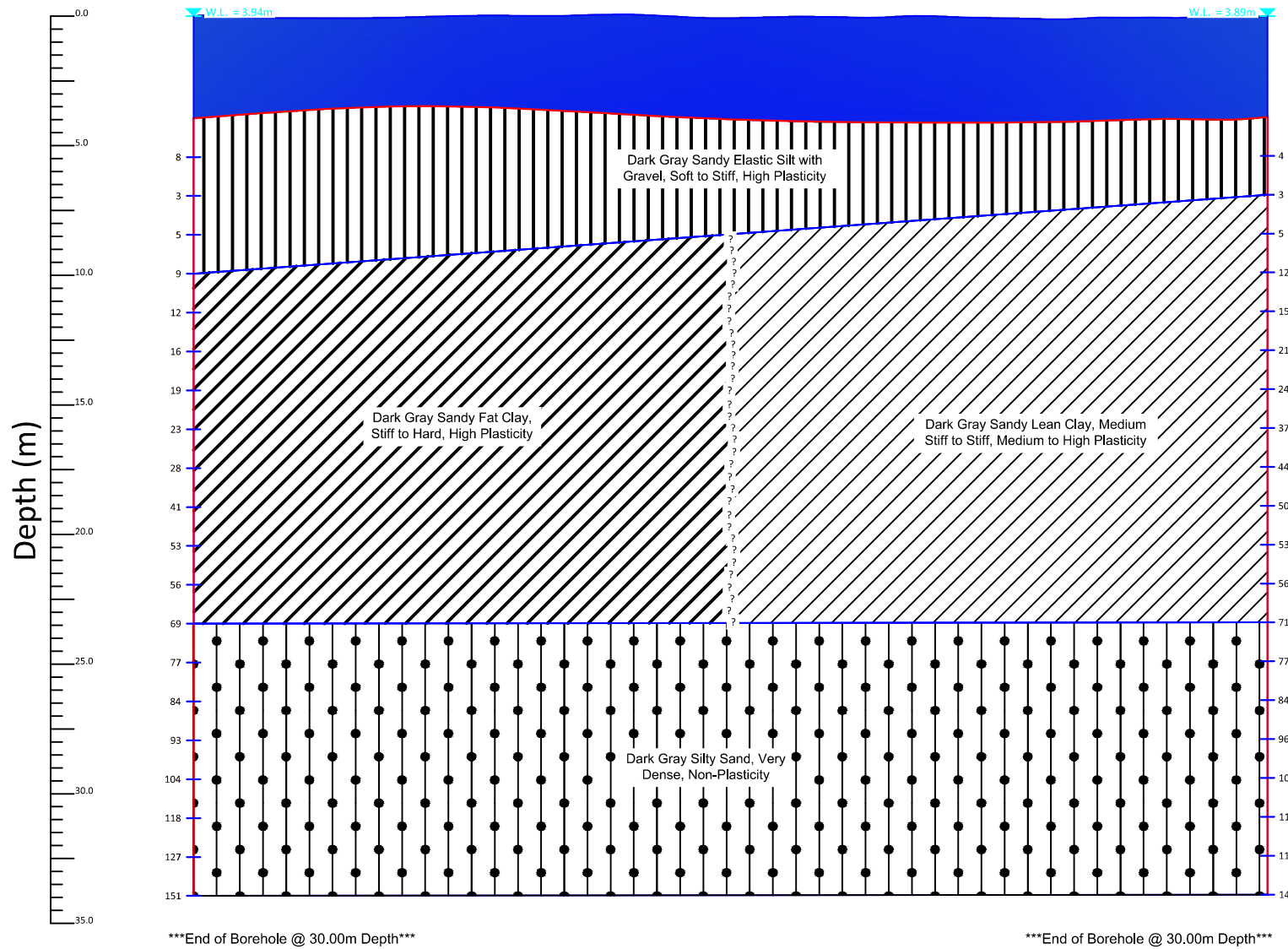
IDEALIZED SOIL PROFILE

BH - 3

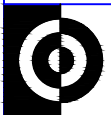
SPT - N Values

BH - 2

SPT - N Values



Typical Soil Sample



GEOSCIENCE TECHNOLOGIES, INC.
 Rm. 207 Makati Executive Tower II, Cityland Square
 Dela Rosa St., Pio del Pilar, Makati City, Tel. No.: (02) 856-3174

PROJECT NAME AND LOCATION :

PROPOSED RESIDENTIAL COMPLEX PROJECT
 Brgy. Coloong #1, Valenzuela City

SHEET CONTENT :

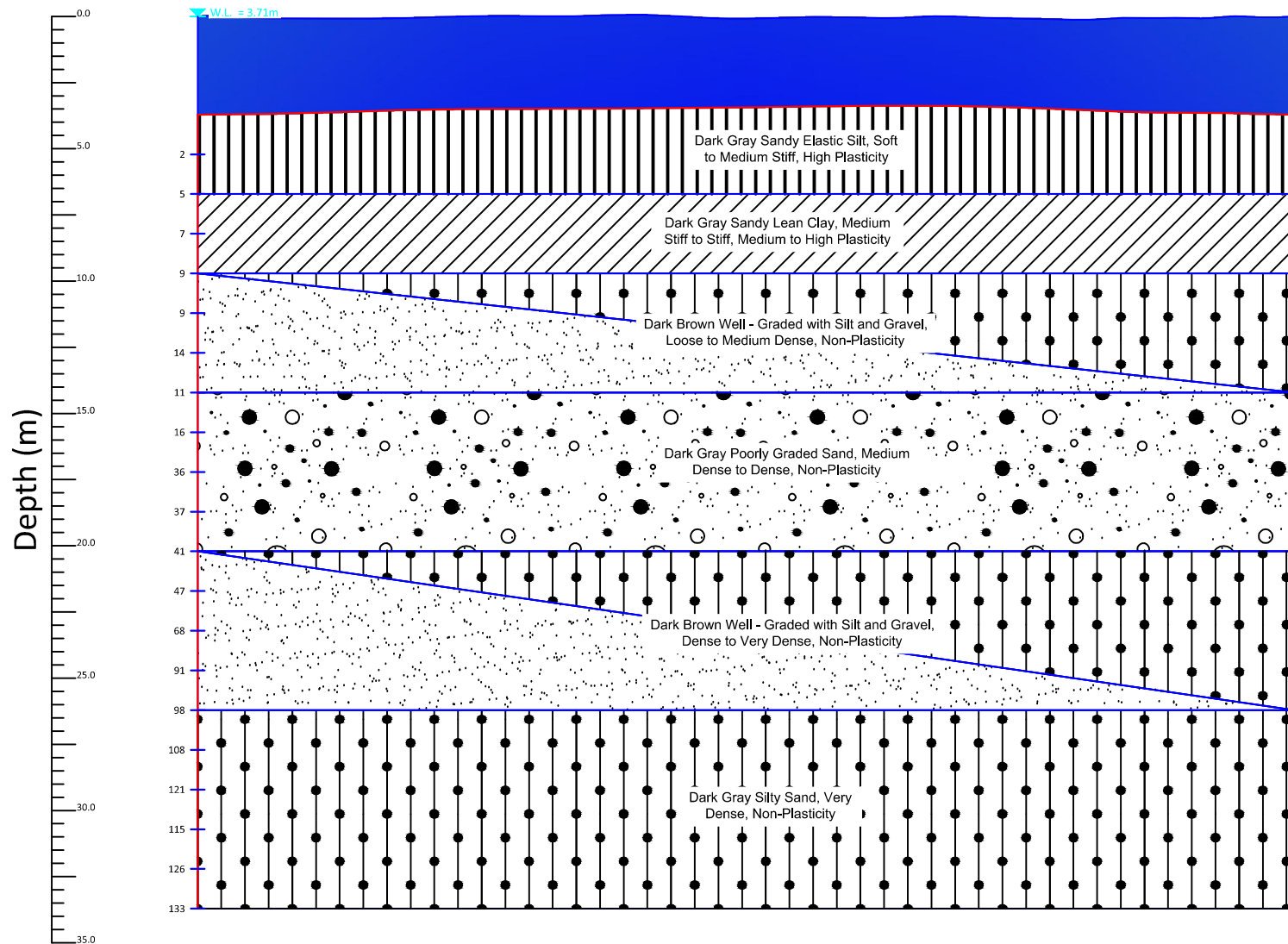
IDEALIZED SOIL PROFILE

SHEET NO.

1 of 2

BH - 1

SPT - N Values



End of Borehole @ 30.00m Depth

Typical Soil Sample



GEOSCIENCE TECHNOLOGIES, INC.
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PROJECT NAME AND LOCATION :

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SHEET CONTENT :

IDEALIZED SOIL PROFILE

SHEET NO.

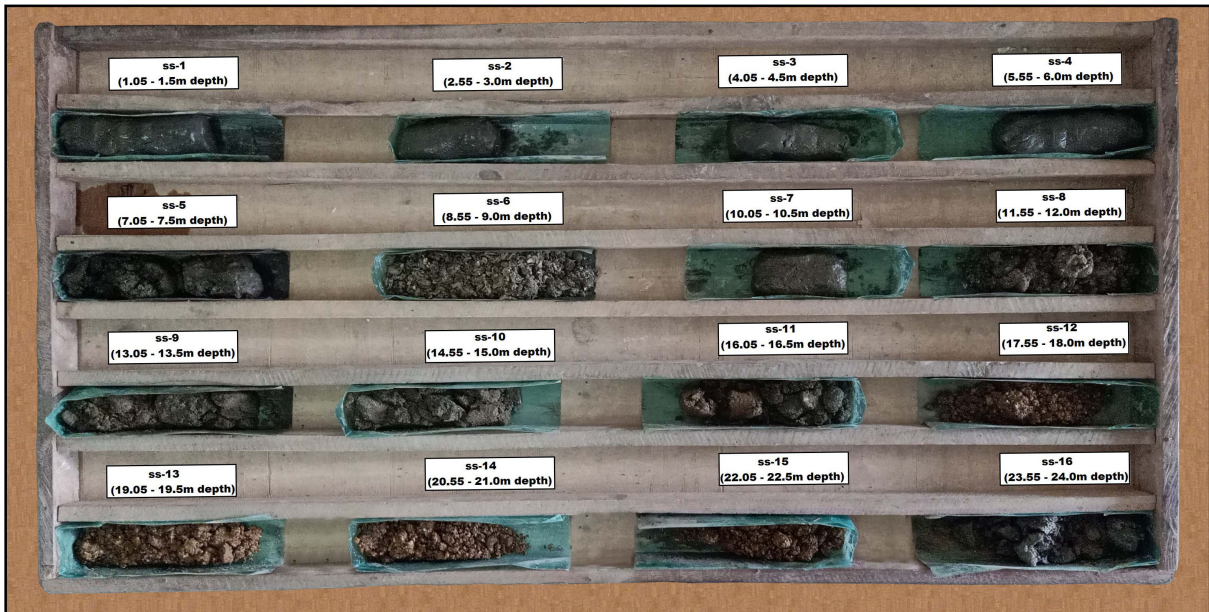
2 of 2

PHOTOGRAPHS



DRILLING WORKS

Borehole No. 1



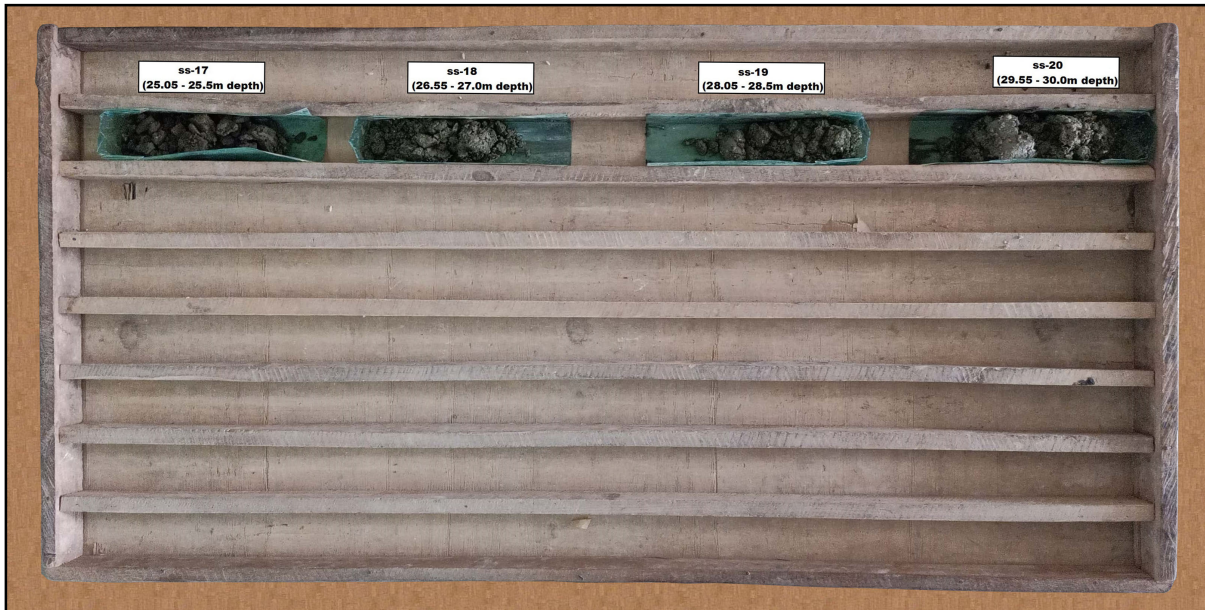
SOIL SAMPLES

Borehole No. 1



DRILLING WORKS

Borehole No. 2



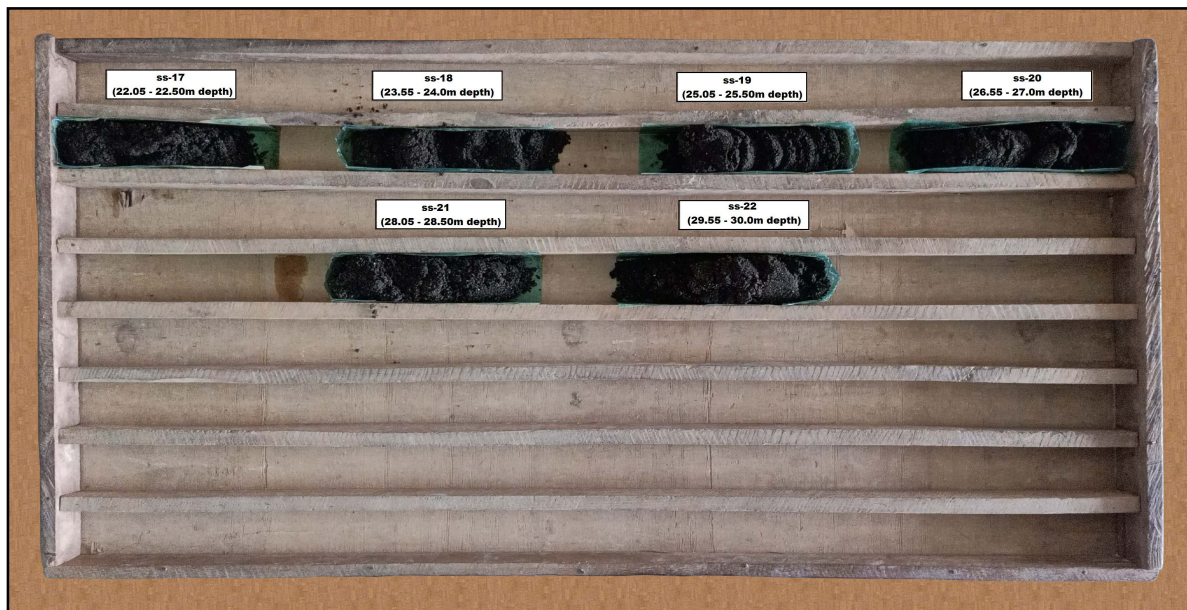
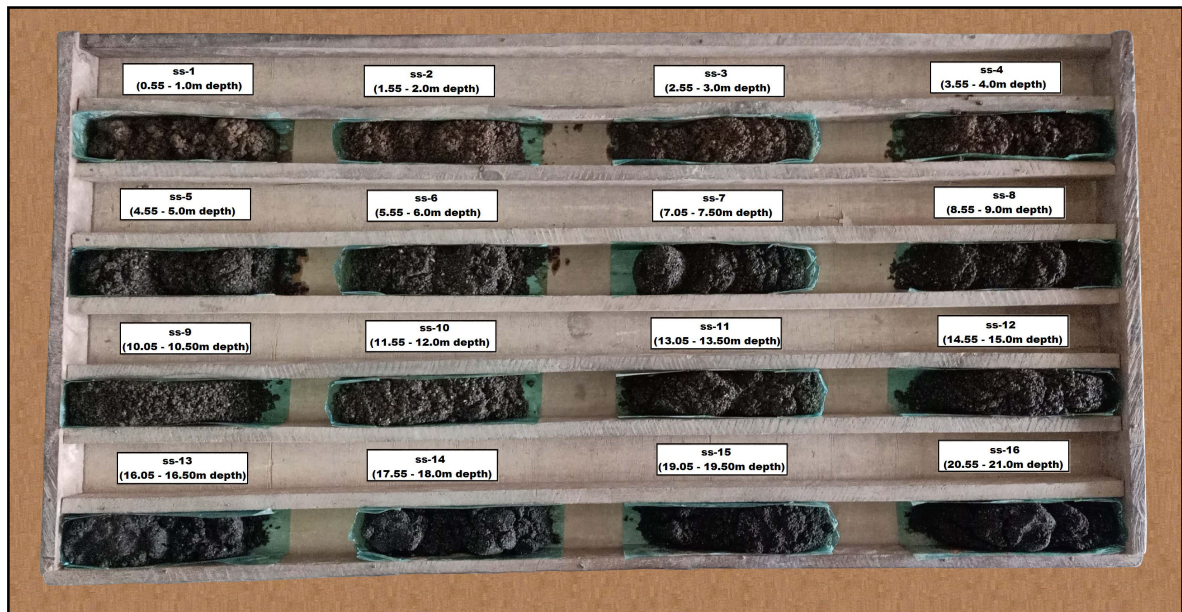
SOIL SAMPLES

Borehole No. 2



DRILLING WORKS

Borehole No. 3



SOIL SAMPLES

Borehole No. 3

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : PROPOSED RESIDENTIAL COMPLEX PROJECT				Final Depth : <u>30.00</u>	Prepared By : <u>GTI</u>
LOCATION : Brgy. Coloong #1, Valenzuela City				Elevation (m) : <u>73.726</u>	Checked By : <u>E.A.A</u>
CLIENT : Technical Assistance Movement for People and Environment (Tampel)		Date Drilled : <u>05/02 - 06/22</u>	Northing : <u>1673489.577</u>		
Borehole No : <u>BH-01</u>	H2O Level bel G.S. (m) : <u>+ 3.71 m</u>	Date Gauged : <u>05/07/2022</u>	Easting : <u>457538.680</u>		
				Sheet No : <u>1 of 2</u>	

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.	
									Liquid Limit	Plasticity index	4	10	40	100	200		Qu(MPa)	Strain at failure E _r (%)
1.50	ss-1	100	-		MH	Dark Gray Sandy Elastic Silt, Soft to Medium Stiff, High Plasticity	BL: 1-1-1 2											
3.00	ss-2	89	-				BL: 2-3-2 5											
4.50	ss-3	89	-		CL	Dark Gray Sandy Lean Clay, Medium Stiff to Stiff, Medium to High Plasticity	BL: 3-3-4 7											
6.00	ss-4	100	-				BL: 6-4-5 9											
7.50	ss-5	89	-				BL: 9-4-5 9											
9.00	ss-6	89	-		SM SW	Dark Brown Well - Graded with Silt and Gravel, Loose to Medium Dense, Non-Plasticity	BL: 7-6-8 14	21.9	NP	NP	74	42	16	8	5.9	2.715		
10.50	ss-7	89	-				BL: 12-5-6 11											
12.00	ss-8	89	-				BL: 11-7-9 16											
13.50	ss-9	89	-		SP	Dark Gray Poorly Graded Sand, Medium Dense to Dense, Non-Plasticity	36 BL: 15-17-19											
15.00	ss-10	89	-				BL: 18-17-20 37	26.7	NP	NP	99	99	95	8	4.5	2.630		

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK					
COHESIVE SOILS (Fine-Grained Soils)		COHESIONLESS SOILS (Coarse-Grained Soils)		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING		Sandy / Silty CLAY (CL) Well-Graded GRAVELS (GW)					
N-VALUE < 2 2 - 4 4 - 8 8 - 15 15 - 30 > 30	CLASSIFICATION VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	N-VALUE < 4 4 - 10 10 - 30 30 - 50 > 50	CLASSIFICATION VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	Organic CLAY (CH) Silty SANDS (SM) Well-Graded SANDS (SW) Poorly Graded SANDS (SP)							
MOISTURE CONTENT (MC)		ROCK QUALITY DESIGNATION (RQD)		PERCENTAGE (% of Sand & Gravel)		Sandy / Clayey / Organic SILTS (ML) Clayey SANDS (SC) SANDSTONE LIMESTONE TUFF BASALT					
RANGES 0 - 10 10 - 30 30 - 70 70 - 100 15 - 30 > 100	VALUE DRY MOIST VERY MOIST WET VERY STIFF SATURATED	RQD (%) < 25 25 - 50 50 - 75 75 - 90 90 - 100	ROCK QUALITY VERY POOR POOR FAIR GOOD EXCELLENT	RANGES (%) 0 - 5 6 - 10 11 - 25 26 - 35 36 - 45	DESCRIPTION TRACES FEW LITTLE SOME WITH						



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : PROPOSED RESIDENTIAL COMPLEX PROJECT				Final Depth : <u>30.00</u>	Prepared By : <u>GTI</u>
LOCATION : Brgy. Coloong #1, Valenzuela City				Elevation (m) : <u>73.726</u>	Checked By : <u>E.A.A</u>
CLIENT : Technical Assistance Movement for People and Environment (Tampel)		Date Drilled : <u>05/02-06/22</u>	Northing : <u>1673489.577</u>		
Borehole No : <u>BH-01</u>	H2O Level bel G.S. (m) : <u>+ 3.71 m</u>	Date Gauged : <u>05/07/2022</u>	Easting : <u>457538.680</u>		
				Sheet No : <u>2 of 2</u>	

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.
									Liquid Limit	Plasticity index	4	10	40	100	200		
16.5	SS-11	89	-		SP	Dark Gray Poorly Graded Sand, Medium Dense to Dense, Non-Plasticity	BL: 21-19-22 41										
18.0	SS-12	89	-				BL: 20-23-24 47										
19.5	SS-13	89	-		SM SW	Dark Brown Well - Graded with Silt and Gravel, Dense to Very Dense, Non-Plasticity	BL: 29-31-67 68	12.1	NP	NP	73	53	30	16	12	2.799	
21.0	SS-14	89	-				BL: 35-42-49 91										
22.50	SS-15	89	-				BL: 46-48-50 98										
24.0	SS-16	100	-				BL: 52-55-53 108										
25.50	SS-17	100	-				BL: 56-59-62 121										
27.00	SS-18	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 58-55-60 115										
28.50	SS-19	89	-				BL: 64-61-65 126										
30.00	SS-20	89	-				BL: 68-66-67 133										

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK					
COHESIVE SOILS (Fine-Grained Soils)		COHESIONLESS SOILS (Coarse-Grained Soils)		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING		Sandy / Silty CLAY (CL)				Well-Graded GRAVELS (GW)	
N-VALUE < 2 2 - 4 4 - 8 8 - 15 15 - 30 > 30	CLASSIFICATION VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	N-VALUE < 4 4 - 10 10 - 30 30 - 50 > 50	CLASSIFICATION VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	Organic CLAY (CH)				BOULDERS			
				Sandy / Clayey / Organic SILTS (ML)				SILTSTONE			
				Clayey SANDS (SC)				SANDSTONE			
				Silty SANDS (SM)				LIMESTONE			
				Well - Graded SANDS (SW)				TUFF			
				Poorly Graded SANDS (SP)				BASALT			
MOISTURE CONTENT (MC)		ROCK QUALITY DESIGNATION (RQD)		PERCENTAGE (% of Sand & Gravel)							
RANGES 0 - 10 10 - 30 30 - 70 70 - 100 15 - 30 > 100	VALUE DRY MOIST VERY MOIST WET VERY STIFF SATURATED	RQD (%) < 25 25 - 50 50 - 75 75 - 90 90 - 100	ROCK QUALITY VERY POOR POOR FAIR GOOD EXCELLENT	RANGES (%) 0 - 5 6 - 10 11 - 25 26 - 35 36 - 45	DESCRIPTION TRACES FEW LITTLE SOME WITH						



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : PROPOSED RESIDENTIAL COMPLEX PROJECT				Final Depth : <u>30.00</u>	Prepared By : <u>GTI</u>
LOCATION : Brgy. Coloong #1, Valenzuela				Elevation (m) : <u>74.707</u>	Checked By : <u>E.A.A</u>
CLIENT : Technical Assistance Movement for People and Environment (Tampel)		Date Drilled : <u>05/13-15/22</u>	Northing : <u>1673547.066</u>		Sheet No : <u>1 of 2</u>
Borehole No : <u>BH-02</u>	H2O Level bel G.S. (m) : <u>+ 3.89 m</u>	Date Gauged : <u>05/16/2022</u>	Easting : <u>457444.655</u>		

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.	
									Liquid Limit	Plasticity Index	4	10	40	100	200		Qu(MPa)	Strain at failure E _r (%)
1.50	ss-1	89	-		MH	Dark Gray Sandy Elastic Silt, Soft, High Plasticity	BL: 1-2-2 4											
3.00	ss-2	89	-				BL: 3-1-2 3											
4.50	ss-3	100	-			Dark Gray Sandy Lean Clay, Medium Stiff to Stiff, Medium to High Plasticity	BL: 2-2-3 5	54.4	40	18	90	79	69	55	40	2.599		
6.00	ss-4	89	-				BL: 5-6-6 12											
7.50	ss-5	89	-				15 BL: 6-7-8											
9.00	ss-6	89	-		CL	Dark Gray Sandy Lean Clay, Very Stiff to Hard, Medium to High Plasticity	BL: 9-11-10 21											
10.50	ss-7	89	-				24 BL: 8-12-12											
12.00	ss-8	89	-				BL: 12-18-19 37											
13.50	ss-9	89	-			Dark Gray Sandy Lean Clay, Very Stiff to Hard, Medium Plasticity	BL: 17-21-23 44											
15.00	ss-10	100	-				BL: 19-24-26 50											

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK					
COHESIVE SOILS (Fine-Grained Soils)		COHESIONLESS SOILS (Coarse-Grained Soils)		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING		Sandy / Silty CLAY (CL)				Well-Graded GRAVELS (GW)	
N-VALUE < 2 2-4 4-8 8-15 15-30 > 30	CLASSIFICATION VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	N-VALUE < 4 4-10 10-30 30-50 > 50	CLASSIFICATION VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	Organic CLAY (CH)				BOULDERS			
				Sandy / Clayey / Organic SILTS (ML)				SILTSTONE			
				Clayey SANDS (SC)				SANDSTONE			
				Silty SANDS (SM)				LIMESTONE			
				Well-Graded SANDS (SW)				TUFF			
				Poorly Graded SANDS (SP)				BASALT			
MOISTURE CONTENT (MC)		ROCK QUALITY DESIGNATION (RQD)		PERCENTAGE (% of Sand & Gravel)							
RANGES 0-10 10-30 30-70 70-100 15-30 > 100	VALUE DRY MOIST VERY MOIST WET VERY STIFF SATURATED	RQD (%) < 25 25-50 50-75 75-90 90-100	ROCK QUALITY VERY POOR POOR FAIR GOOD EXCELLENT	RANGES (%) 0-5 6-10 11-25 26-35 36-45	DESCRIPTION TRACES FEW LITTLE SOME WITH						



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : PROPOSED RESIDENTIAL COMPLEX PROJECT
LOCATION : Brgy. Coloong #1, Valenzuela City
CLIENT : Technical Assistance Movement for People and Environment (Tampel)

Final Depth : 30.00
 Elevation (m) : 74.707
 Northing : 1673547.066
 Easting : 457444.655

Prepared By : GTI
 Checked By : E.A.A
 Sheet No : 2 of 2

Borehole No : BH-02 H2O Level bel G.S. (m) : + 3.89 m Date Drilled : 05/13-15/22
 Date Gauged : 05/16/2022

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.
									Liquid Limit	Plasticity Index	4	10	40	100	200		
16.5	SS-11	89	-		CL	Dark Gray Sandy Lean Clay, Very Stiff to Hard, Medium Plasticity	BL: 21-25-28 53	35.3	NP	NP	99	94	85	79	64	2.644	
18.0	SS-12	100	-				BL: 23-27-29 56										
19.5	SS-13	100	-				BL: 28-32-39 71										
21.0	SS-14	100	-				BL: 34-37-40 77										
22.50	SS-15	89	-				BL: 39-41-43 84										
24.0	SS-16	89	-				BL: 44-47-49 96										
25.50	SS-17	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 46-49-51 100										
27.00	SS-18	89	-				BL: 53-56-56 112	59.5	NP	NP	94	78	44	23	17	2.649	
28.50	SS-19	89	-				BL: 57-59-59 118										
30.00	SS-20	89	-				BL: 61-64-78 142										

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK			
COHESIVE SOILS (Fine-Grained Soils) N-VALUE CLASSIFICATION < 2 VERY SOFT 2 - 4 SOFT 4 - 8 MEDIUM STIFF 8 - 15 STIFF 15 - 30 VERY STIFF > 30 HARD		COHESIONLESS SOILS (Coarse-Grained Soils) N-VALUE CLASSIFICATION < 4 VERY LOOSE 4 - 10 LOOSE 10 - 30 MEDIUM DENSE 30 - 50 DENSE > 50 VERY DENSE		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING 		Sandy / Silty CLAY (CL) Organic CLAY (CH) Sandy / Clayey / Organic SILTS (ML) Clayey SANDS (SC) Silty SANDS (SM) Well - Graded SANDS (SW) Poorly Graded SANDS (SP) Well - Graded GRAVELS (GW) BOULDERS SILTSTONE SANDSTONE LIMESTONE TUFF BASALT 			
MOISTURE CONTENT (MC) RANGES VALUE 0 - 10 DRY 10 - 30 MOIST 30 - 70 VERY MOIST 70 - 100 WET 15 - 30 VERY STIFF > 100 SATURATED		ROCK QUALITY DESIGNATION (RQD) RQD (%) ROCK QUALITY < 25 VERY POOR 25 - 50 POOR 50 - 75 FAIR 75 - 90 GOOD 90 - 100 EXCELLENT		PERCENTAGE (% of Sand & Gravel) RANGES (%) DESCRIPTION 0 - 5 TRACES 6 - 10 FEW 11 - 25 LITTLE 26 - 35 SOME 36 - 45 WITH					



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : PROPOSED RESIDENTIAL COMPLEX PROJECT
LOCATION : Brgy. Coloong #1, Valenzuela City
CLIENT : Technical Assistance Movement for People and Environment (Tampel) Date Drilled : 05/21-23/22
 Borehole No : BH-03 H2O Level bel G.S. (m) : + 3.94 m Date Gauged : 05/24/2022
 Final Depth : 30.00
 Elevation (m) : 74.707
 Northing : 1673547.066
 Easting : 457444.655
 Prepared By : GTI
 Checked By : E.A.A
 Sheet No : 1 of 2

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.	
									Liquid Limit	Plasticity Index	4	10	40	100	200			Qu(MPa)
1.50	ss-1	89	-		MH	Dark Gray Sandy Elastic Silt with Gravel, Soft to Stiff, High Plasticity	BL: 1-1-2 3	86.9	50	16	81	63	48	45	43	2.600		
3.00	ss-2	89	-				BL: 2-1-2 3											
4.50	ss-3	89	-				BL: 2-2-3 5											
6.00	ss-4	89	-				BL: 5-4-5 9											
7.50	ss-5	89	-		CH	Dark Gray Sandy Fat Clay, Stiff to Hard, High Plasticity	BL: 4-6-6 12											
9.00	ss-6	100	-				BL: 6-7-9 16											
10.50	ss-7	89	-				BL: 8-9-10 19											
12.00	ss-8	100	-				BL: 9-11-1 23											
13.50	ss-9	89	-				28 BL: 11-13-15	73.6	82	49	96	92	88	87	86	2.610		
15.00	ss-10	89	-			BL: 16-19-22 41												

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK			
COHESIVE SOILS (Fine-Grained Soils)		COHESIONLESS SOILS (Coarse-Grained Soils)		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING		Sandy / Silty CLAY (CL) Organic CLAY (CH) Sandy / Clayey / Organic SILTS (ML) Clayey SANDS (SC) Silty SANDS (SM) Well - Graded SANDS (SW) Poorly Graded SANDS (SP)			
N-VALUE < 2 2 - 4 4 - 8 8 - 15 15 - 30 > 30	CLASSIFICATION VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	N-VALUE < 4 4 - 10 10 - 30 30 - 50 > 50	CLASSIFICATION VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	PERCENTAGE (% of Sand & Gravel) RANGES (%) DESCRIPTION 0 - 5 TRACES 6 - 10 FEW 11 - 25 LITTLE 26 - 35 SOME 36 - 45 WITH		Well - Graded GRAVELS (GW) BOULDERS SILTSTONE SANDSTONE LIMESTONE TUFF BASALT			
MOISTURE CONTENT (MC) RANGES VALUE 0 - 10 DRY 10 - 30 MOIST 30 - 70 VERY MOIST 70 - 100 WET 15 - 30 VERY STIFF > 100 SATURATED		ROCK QUALITY DESIGNATION (RQD) RQD (%) ROCK QUALITY < 25 VERY POOR 25 - 50 POOR 50 - 75 FAIR 75 - 90 GOOD 90 - 100 EXCELLENT							



GEOSCIENCE TECHNOLOGIES INC.

FINAL BOREHOLE LOG AND SUMMARY OF TEST RESULTS

PROJECT : **PROPOSED RESIDENTIAL COMPLEX PROJECT**
 LOCATION : **Brgy. Coloong #1, Valenzuela City**
 CLIENT : **Technical Assistance Movement for People and Environment (Tampel)** Date Drilled : 05/21-23/22
 Borehole No : BH-03 H2O Level bel G.S. (m) : + 3.94 m Date Gauged : 05/24/2022

Final Depth : 30.00
 Elevation (m) : 74.707
 Northing : 1673547.066
 Easting : 457444.655

Prepared By : GTI
 Checked By : E.A.A
 Sheet No : 2 of 2

DEPTH, m.	Sample No.	Recovery Sample %	R.O.D. %	Log Symbol	Soil classification	DESCRIPTION	N - BLOWS	Natural Moisture Content	Atterberg Limits		Sieve Analysis % Passing					Specific Gravity	Other Test U.C.T.	
									Liquid Limit	Plasticity Index	4	10	40	100	200		Qu(MPa)	Strain at failure E _r (%)
16.5	SS-11	89	-		CH	Dark Gray Sandy Fat Clay, Hard, High Plasticity	BL: 26-24-29 53											
18.0	SS-12	89	-		CH	Dark Gray Sandy Fat Clay, Hard, High Plasticity	BL: 24-25-31 56											
19.5	SS-13	89	-		CH	Dark Gray Sandy Fat Clay, Hard, High Plasticity	BL: 27-31-38 69											
21.0	SS-14	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 32-35-42 77											
22.50	SS-15	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 36-40-44 84	28.9	NP	NP	94	75	36	24	21	2.672		
24.0	SS-16	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 40-45-48 93											
25.50	SS-17	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 47-50-54 104											
27.00	SS-18	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 52-57-61 118											
28.50	SS-19	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 59-62-65 127											
30.00	SS-20	89	-		SM	Dark Gray Silty Sand, Very Dense, Non-Plasticity	BL: 64-72-79 151											

CONSISTENCY				TYPE OF SAMPLING		TYPE OF SOIL & ROCK			
COHESIVE SOILS (Fine-Grained Soils)		COHESIONLESS SOILS (Coarse-Grained Soils)		WASH BORING (WB) STANDARD PENETRATION TEST (SPT) UNDISTURBED SAMPLING (UDS) CORING		Sandy / Silty CLAY (CL) Organic CLAY (CH) Sandy / Clayey / Organic SILTS (ML) Clayey SANDS (SC) Silty SANDS (SM) Well - Graded SANDS (SW) Poorly Graded SANDS (SP)			
N-VALUE < 2 2 - 4 4 - 8 8 - 15 15 - 30 > 30	CLASSIFICATION VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	N-VALUE < 4 4 - 10 10 - 30 30 - 50 > 50	CLASSIFICATION VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	Well - Graded GRAVELS (GW) BOULDERS SILTSTONE SANDSTONE LIMESTONE TUFF BASALT					
MOISTURE CONTENT (MC) RANGES VALUE		ROCK QUALITY DESIGNATION (RQD) RQD (%) ROCK QUALITY		PERCENTAGE (% of Sand & Gravel) RANGES (%) DESCRIPTION					
0 - 10	DRY	< 25	VERY POOR	0 - 5	TRACES				
10 - 30	MOIST	25 - 50	POOR	6 - 10	FEW				
30 - 70	VERY MOIST	50 - 75	FAIR	11 - 25	LITTLE				
70 - 100	WET	75 - 90	GOOD	26 - 35	SOME				
15 - 30	VERY STIFF	90 - 100	EXCELLENT	36 - 45	WITH				
> 100	SATURATED								

LABORATORY TEST RESULTS

- ❖ Moisture Content Determination
- ❖ Particle-Size Analysis
- ❖ Atterberg Limit
- ❖ Specific Gravity



Report No: PL22-2645A/1

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174


 Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645A
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645A

Sample Submitted By: C. Morris
Source: Jobsite - BH-01
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-01 @ Elev. 9.00m
Sample Description: Soil / Bore Hole
Sample ID: SS-6
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 96
		4.75 74
		2.00 42
		0.850 22
		0.425 16
0.250 11		
0.150 8		
0.075 5.9		
ASTM D2216	Moisture Content %	21.9
ASTM C117	Material Finer Than No. 200 Sieve %	5.9
ASTM D4318	Liquid Limit	Non Plastic
ASTM D4318	Plastic Limit	Non Plastic
ASTM D4318	Plasticity Index	Non Plastic
ASTM D854	Specific Gravity of Soil	2.715
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-1-a
ASTM D2487	Unified Soil Classification System	SW-SM - Well-graded sand with silt and gravel

Tested by:


 A.M. Francisco

Sr. Laboratory Technician

Checked by:


 M.G. Guevarra

Laboratory Supervisor

Remarks



Report No: PL22-2645A/2

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022	Sample Submitted By: C. Morris
Job Order ID: 22-2380	Source: Jobsite - BH-01
Lab. Ref. ID: 2205-SOI-2645A	Sampled By: Geoscience Technologies, Inc.
Date Tested: May 25, 2022 to May 27, 2022	Sampled at: BH-01 @ Elev. 15.00m
Test No.: 220525-QT-2645A	Sample Description: Soil / Bore Hole
	Sample ID: SS-10
	Blows:
Date Submitted: May 25, 2022	
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.	

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 100
		4.75 99
		2.00 99
		0.850 98
	0.425 95	
	0.250 47	
	0.150 8	
	0.075 4.5	
ASTM D2216	Moisture Content %	26.7
ASTM C117	Material Finer Than No. 200 Sieve %	4.5
ASTM D4318	Liquid Limit	Non Plastic
ASTM D4318	Plastic Limit	Non Plastic
ASTM D4318	Plasticity Index	Non Plastic
ASTM D854	Specific Gravity of Soil	2.630
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-3
ASTM D2487	Unified Soil Classification System	SP - Poorly graded sand

Tested by:

A.M. Francisco

Sr. Laboratory Technician

Checked by:

M.G. Guevarra

Laboratory Supervisor

Remarks



Report No: PL22-2645A/3

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signature: Reynaldo Guevarra
 General Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645A
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645A

Sample Submitted By: C. Morris
Source: Jobsite - BH-01
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-01 @ Elev. 19.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-13
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 93
		9.5 87
		4.75 73
		2.00 53
		0.850 38
		0.425 30
0.250 21		
0.150 16		
0.075 12		
ASTM D2216	Moisture Content %	12.1
ASTM C117	Material Finer Than No. 200 Sieve %	11.6
ASTM D4318	Liquid Limit	Non Plastic
ASTM D4318	Plastic Limit	Non Plastic
ASTM D4318	Plasticity Index	Non Plastic
ASTM D854	Specific Gravity of Soil	2.799
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-1-b
ASTM D2487	Unified Soil Classification System	SW-SM - Well-graded sand with silt and gravel

Tested by:

A.M. Francisco
 Sr. Laboratory Technician

Checked by:

M.G. Guevarra
 Laboratory Supervisor

Remarks




Report No: PL22-2645B/1

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174


 Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645B
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645B

Sample Submitted By: C. Morris
Source: Jobsite - BH-02
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-02 @ Elev. 4.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-3
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 97
		4.75 90
		2.00 79
		0.850 72
		0.425 69
0.250 65		
0.150 55		
0.075 40		
ASTM D2216	Moisture Content %	54.4
ASTM C117	Material Finer Than No. 200 Sieve %	39.8
ASTM D4318	Liquid Limit	40
ASTM D4318	Plastic Limit	22
ASTM D4318	Plasticity Index	18
ASTM D854	Specific Gravity of Soil	2.599
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-6 (3)
ASTM D2487	Unified Soil Classification System	CL - Sandy Lean Clay

Tested by:


 A.M. Francisco

Sr. Laboratory Technician

Checked by:


 M.G. Guevarra
 Laboratory Supervisor

Remarks



Report No: PL22-2645B/2

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

The content of this report is governed by our standard terms and conditions and may not be reproduced other than in full. Where our involvement consists exclusively of testing samples, the results relate only to the samples tested.

Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645B
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645B

Sample Submitted By: C. Morris
Source: Jobsite - BH-02
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-02 @ Elev. 16.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-11
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 100
		4.75 99
		2.00 94
		0.850 87
		0.425 85
0.250 83		
0.150 79		
0.075 64		
ASTM D2216	Moisture Content %	35.3
ASTM C117	Material Finer Than No. 200 Sieve %	64.4
ASTM D4318	Liquid Limit	38
ASTM D4318	Plastic Limit	26
ASTM D4318	Plasticity Index	12
ASTM D854	Specific Gravity of Soil	2.644
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-6 (7)
ASTM D2487	Unified Soil Classification System	CL - Sandy Lean Clay

Tested by:

A.M. Francisco

Sr. Laboratory Technician

Checked by:

M.G. Guevarra

Laboratory Supervisor

Remarks



Report No: PL22-2645B/3

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645B
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645B

Sample Submitted By: C. Morris
Source: Jobsite - BH-02
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-02 @ Elev. 27.00m
Sample Description: Soil / Bore Hole
Sample ID: SS-18
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 100
		4.75 94
		2.00 78
		0.850 58
		0.425 44
0.250 30		
0.150 23		
0.075 17		
ASTM D2216	Moisture Content %	59.5
ASTM C117	Material Finer Than No. 200 Sieve %	17.1
ASTM D4318	Liquid Limit	Non Plastic
ASTM D4318	Plastic Limit	Non Plastic
ASTM D4318	Plasticity Index	Non Plastic
ASTM D854	Specific Gravity of Soil	2.649
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-1-b
ASTM D2487	Unified Soil Classification System	SM - Silty Sand

Tested by:

A.M. Francisco

Sr. Laboratory Technician

Checked by:

M.G. Guevarra

Laboratory Supervisor

Remarks



Report No: PL22-2645C/1

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645C
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645C

Sample Submitted By: C. Morris
Source: Jobsite - BH-03
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-03 @ Elev. 1.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-1
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 92
		4.75 81
		2.00 63
		0.850 51
		0.425 48
0.250 46		
0.150 45		
0.075 43		
ASTM D2216	Moisture Content %	86.9
ASTM C117	Material Finer Than No. 200 Sieve %	42.7
ASTM D4318	Liquid Limit	50
ASTM D4318	Plastic Limit	34
ASTM D4318	Plasticity Index	16
ASTM D854	Specific Gravity of Soil	2.600
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-7-5 (4)
ASTM D2487	Unified Soil Classification System	MH - Sandy Elastic Silt with Gravel

Tested by:

A.M. Francisco
 Sr. Laboratory Technician

Checked by:

M.G. Guevarra
 Laboratory Supervisor

Remarks



Report No: PL22-2645C/2

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645C
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645C

Sample Submitted By: C. Morris
Source: Jobsite - BH-03
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-03 @ Elev. 13.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-9
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 96
		9.5 96
		4.75 96
		2.00 92
		0.850 90
		0.425 88
0.250 88		
0.150 87		
0.075 86		
ASTM D2216	Moisture Content %	73.6
ASTM C117	Material Finer Than No. 200 Sieve %	85.7
ASTM D4318	Liquid Limit	82
ASTM D4318	Plastic Limit	33
ASTM D4318	Plasticity Index	49
ASTM D854	Specific Gravity of Soil	2.640
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-7-5 (48)
ASTM D2487	Unified Soil Classification System	CH - Sandy Fat Clay

Tested by:

A.M. Francisco

Sr. Laboratory Technician

Checked by:

M.G. Guevarra

Laboratory Supervisor

Remarks



Report No: PL22-2645C/3

Issue No : 1

Test Report on Soil Test

Client: Geoscience Technologies, Inc.
Address: Rm. 207 Makati Executive Tower II, Cityland Square,
 Dela Rosa St., Pio del Pilar, Makati City
Project Name: Proposed Residential Complex
Project Location: Brgy. Coloong #1, Valenzuela
Contractor: Geoscience Technologies, Inc.
Tel / Fax No: 02-856-3174

Approved Signatory: Reynaldo Guevarra
 General/ Technical Manager
 Date of Issue: May 27, 2022

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Sample Details

Received Date: May 25, 2022
Job Order ID: 22-2380
Lab. Ref. ID: 2205-SOI-2645C
Date Tested: May 25, 2022 to May 27, 2022
Test No.: 220525-QT-2645C

Sample Submitted By: C. Morris
Source: Jobsite - BH-03
Sampled By: Geoscience Technologies, Inc.
Sampled at: BH-03 @ Elev. 22.50m
Sample Description: Soil / Bore Hole
Sample ID: SS-15
Blows:

Date Submitted: May 25, 2022
Test Desired: Sieve Analysis, Natural Moisture Content, Atterberg Limit, Specific Gravity and Soil Classification Test.

Test Results

Method	Test / Parameter	Result
ASTM D6913	Particle Size Distribution	Sieve Size (mm) % Passing
		50.0 100
		37.5 100
		25.0 100
		19.0 100
		12.5 100
		9.5 100
		4.75 94
		2.00 75
		0.850 48
0.425 36		
0.250 28		
0.150 24		
0.075 21		
ASTM D2216	Moisture Content %	28.9
ASTM C117	Material Finer Than No. 200 Sieve %	20.6
ASTM D4318	Liquid Limit	Non Plastic
ASTM D4318	Plastic Limit	Non Plastic
ASTM D4318	Plasticity Index	Non Plastic
ASTM D854	Specific Gravity of Soil	2.672
ASTM D3282	Soil and Soil-Aggregates Mixtures Classification	A-1-b
ASTM D2487	Unified Soil Classification System	SM - Silty Sand

Tested by:

A.M. Francisco

Sr. Laboratory Technician

Checked by:

M.G. Guevarra

Laboratory Supervisor

Remarks